MONROE COUNTY DRAINAGE BOARD

Tuesday August 13, 2024, at 8:30 AM Location: Showers Building Room 106D Hybrid Meeting with Virtual Attendance via Teams

AGENDA

- 1. Call to Order
- 2. Approval of Minutes for: May 14, 2024 +*
- 3. Public Input for Items not on the Agenda
- 4. Business
 - a. Preliminary Drainage Plan: Cutright Boat Storage +*
 - b. Residential Drainage Plan: Fry Rd. +*
 - c. Enforcement Case: Burke, Ison Rd +*
- 5. Staff Reports
- 6. Adjournment
 - a. Date of Next Meeting: Tuesday September 10, 2024, at 8:30 AM
- + Attachment Included
- * Board Action Requested

The meeting link and packet will be available on the County Events Calendar at www.co.monroe.in.us.

Anyone who requires an auxiliary aid or service for effective communication, or a modification of policies or procedures to participate in a program, service, or activity of Monroe County, should contact Monroe County Title VI Coordinator Angie Purdie, (812)349-2550, <u>apurdie@co.monroe.in.us</u>, as soon as possible but no later than forty-eight (48) hours before the scheduled event.

Individuals requiring special language services should, if possible, contact the Monroe County Government Title VI Coordinator at least seventy-two (72) hours prior to the date on which the services will be needed.

The meeting is open to the public.

MEETING MINUTES MONROE COUNTY DRAINAGE BOARD Thursday, May 14, 2024 at 8:30 AM Showers Building Room 106D Hybrid Meeting with Virtual Attendance via Zoom

MEMBERS PRESENT: Robert Autio, Ginger Davis, Terry Quillman, Trohn Enright-Randolph *(ex officio)*, Lee Jones

MEMBERS ABSENT: Bill Riggert

STAFF: Kelsey Thetonia (MS4 Coordinator), Donna Barbrick (Secretary), Tina Engle (Stormwater), Tammy Behrman (Planning), David Schilling (Legal), Lisa Ridge (Highway), TSD

OTHERS: Katie Stein (Bynum Fanyo), Dillon Reynolds (Spaceco), Madeline Romeo (Spaceco)

1. Call to Order by Robert Autio.

2. Approval of Minutes for: February 1, 2024, and March 7, 2024 Motion to approve the minutes by Lee Jones; second by Terry Quillman. VOTE: AYE

(unanimous). Motion carried.

3. Public Input for Items not on the Agenda: none

4. Business

a. Preliminary Drainage Plan: Bloomington Country Club Improvements +* Kelsey Thetonia gave an overview of the site. She said the whole area drains to sinkholes no matter what direction you go. She said we are looking at critical drainage area release rates, water quality treatment and velocity dissipation. She said we will ask for drainage easements and a sinkhole conservancy area.

(Trohn Enright-Randolph arrived to the meeting.)

Thetonia said there is an active sinkhole on the site and the country club is working with a hydrogeologist on mitigation. She talked about possible methods of mitigation. Katie Stein said in 2020 they received a building permit for some site improvements. She said there was no water quality or detention provided at that time. She said they would like to do improvements to their pool area. She said the whole area that would be disturbed is less than an acre. She said the focus is on slowing down the water to provide water quality.

Quillman had a question about the drainage basin. Stein said it is a small basin and runoff rates would be reduced with the proposed plan. Thetonia said she would talk to Jason (hydrogeologist) about doing some type of swale on the east side of the parking area that can help slow the water down before it gets to the detention pond. Autio suggested a motion to approve the preliminary drainage plan. Quillman said he would make that motion with the listed conditions; seconded by Autio. VOTE: YES (unanimous). Motion carried.

b. Preliminary Drainage Plan: U-Haul in Pinnacle Business Park +* Thetonia said the engineer was attending virtually. She said this is the second large development in this area. She said everything drains south to regional ponds. She referenced a couple of studies that were included in the packet. She said we are requesting water quality treatment for this. She said they would be developing two lots, Lots #5 and #6. She gave an overview of the site and invited the engineers to unmute.

Dillon Reynolds said Madeline Romeo was also online. He gave a quick rundown of the project for U-Haul, with multiple buildings on site including storage space and showroom space. He said there are two lots with the drainage easement in between. He said the proposal is making this one lot and removing the drainage easement down the middle. He said we are looking at water quality. He said the proposal includes a detention pond and some hydrodynamic separators before the water goes to the pond.

Ginger Davis had a question about offsite drainage going through the site. Reynolds said we would keep it going to the west. He noted a northwest corner that would not be developed. Thetonia gave an overview of where affected soils are located and Reynolds brought up the plat showing those. Davis had a question about an outlet going into the affected area. Reynolds said we will have riprap protection.

Quillman talked about an area by the railroad track. Lisa Ridge commented said the county bought the area as a right of way because it was undeveloped.

Trohn Enright-Randolph said it is kind of tight looking at the elevations and the contours. Thetonia said we will make sure in inspection that they have followed the final drainage plan. Reynolds said the easement was placed on the lot line, but the ultimate outlet would still be the pond. He said there is no infrastructure or defined swale currently. Thetonia said I will ask that you reach out to the Highway Department to plans for Sunrise Greeting Court extensions and so that their final grades are in their plans as well as any storm sewers in that area, so that you can make sure to account for any of those changes. She said that project is currently under construction.

Thetonia said there is a drainage easement on the southside that goes to an existing driveway. She said they are proposing to fill in a portion of the drainage easement for pavement here. She said in the swales report, there is a high-water elevation that Katie modeled and there will be some fill within that, so you are losing some of that storage by filling this in. Trohn asked why they were encroaching. Reynolds said there is already a curb cut off the road, so we are trying to extend that into our site and that drainage easement goes right to the back of that curb cut. He said so if you want to make it useful, you have to fill in a little bit of this to make the grades work out. He said we are going to look at revising the grading a little bit to not lose any storage that is being used. He said we are also going to see exactly how much pavement we really need and maybe we can do something to allow this area to not be filled as much. He said there is nothing final yet, but we are looking at options.

Motion by Davis to approve with conditions listed including erosion control at the outlets and approval of removal of the drainage easement between the lot lines and conditions relative to reducing the parking area and reducing the fill requirements and reviewing Highway plans for Sunrise Greetings Court extensions. Second by Quillman. VOTE: YES (unanimous). Motion carried.

5. Stormwater Management Ordinance Discussion

Thetonia said Dave Schilling (Legal) has spent a lot of time going over this and we have set the adoption target date as June 5. Davis said I had a few feedback items on the karst presentation. There was a discussion of karst related items in the new ordinance and technical standards. There was a discussion of protected information being included in a karst overlay. Thetonia said that a karst overlay is not specifically mentioned in this ordinance. She said this is specifically talking about sinkhole watershed areas, a larger overlay that is going to include more data.

Julie Thomas joined the meeting via Teams. Penny Githens arrived at approximately 9:55 a.m.

Thetonia gave a summary of the background for why a new ordinance was necessary to keep up with state requirements. She talked about working with Christopher Burke Engineering and taking their model ordinance and making it work for Monroe County. She talked about having a way to work with loggers to make sure that they are implementing BMPs and being able to follow up to make sure that they close out the sites properly and everything is stabilized when they leave.

She said we have a chapter on stormwater permitting requirements and that includes Drainage Board (DB) procedures. She said when someone needs to take a project to the DB that should be in there. She said we can do some administrative enforcement procedures for construction sites like issuing stop work orders, penalties, withholding of permits and things like that to make sure that the requirements of this ordinance are going to be met. She talked about supporting documents including a Stormwater Technical Standards Manual, Stormwater Construction Specifications, and Indiana Logging & Forestry Best Management Practices. She said there is a version of the ordinance on Teams. She asked for comments or questions.

There was a discussion of the DB's role. Thetonia said the DB is referenced in the ordinance as the oversight. She said the DB has a lot of say in one of the major aspects of the ordinance. There was a discussion of rainfall tables and updating tables to reflect changes in climate.

The DB discussed voting to recommend approval of the ordinance with the condition that comments that were brought up today are addressed. **Davis said I'd like to make a recommendation, with the amendments and changes as discussed, barring any out of the ordinary, that we give our recommendation that it move forward to be adopted as an ordinance by the commissioners.** Autio said that is a motion for approval of a positive **recommendation on the new ordinance; I will second that motion.** VOTE by Drainage **Board members present: YES (unanimous).** Motion carried. Trohn said I hope the commissioners will consider amending the chapter and they think it's reasonable to have some recommendation from the DB when the ordinance and technical documents are amended.

There was a discussion of wording about slopes in the planning ordinance draft. Thetonia said the ordinance itself contains the general pollution prevention standards. She said we don't call for a specific slope but rather that you use erosion and controls adequate for your site and based on soil tests. Schilling commented about the ordinance, saying this is a major code revision based a lot on state revisions that are pretty major. He said be ready for tweaks and changes. Thetonia said thank you so much, Dave, for your guidance on this; you've put countless hours into this so thank you.

7. Adjournment

a. Date of Next Meeting: Tuesday June 11, 2024, at 8:30 AM

The meeting adjourned at approximately 10:48 a.m.

Minutes approved: _____

President, Drainage Board

Secretary, Drainage Board

| Project Name: | Cutright Boat Storage PUD |
|-----------------------|---------------------------|
| Engineer/Design Firm: | Banning Engineering |
| Address: | 8370 S State Road 446 |
| Acres: | 15.3 |

Planning Number: Watershed: Karst Report: Wetland Delineation: Not Completed

PUO-24-2 Monroe Lake Not Completed

Project Summary

The Cutright Boat Storage PUD is located on the west side of SR 446 just north of E Allens Creek Road near Lake Monroe. The PUD is surrounded by properties zoned Forest Reserve. This project is proposing to subdivide the existing parcels into three lots. The north lot (4.64 acres) will contain 1.7 acres of boat storage facilities.



Project Drainage

This project is located in the Lake Monroe watershed and drains northwest towards Lake Monroe. There are no mapped floodplains and no known sinkholes on or adjacent to the property.

Cutright Boat Storage Preliminary Drainage Comments:

Detention and Water Quality Treatment:

- This project is in the Lake Monroe watershed. General County release rates will be required for detention (0.45 cfs/acre for 10% EP, 0.9 cfs/acre for 1% EP.
- Water quality treatment is required per Ch. 761. Additional water quality measures may be required due to the proposed landuse. This can be determined during development plan review.
- Grass swale with rock check dams and detention basin are proposed as water quality treatment practices. Adequacy of Outlet:
 - Detention basin will outlet to a rip rap stilling basin for velocity dissipation.

Drainage Easements:

- Drainage easements will be required per Ch. 761.



July 1, 2024

Monroe County Drainage Board C/O Kelsey Thetonia Monroe County Highway Department 5900 W Foster Curry Dr Bloomington, IN 47403

RE; Greemann SR 446 Boat Storage Preliminary Drainage Review

On behalf of our client, Thomas Greemann, we respectfully request to be placed on the agenda for the July 9th Drainage Board meeting for consideration of preliminary approval of a drainage plan for the proposed boat storage project located at 8370 S. State Road 446. Details of the project are shown on the attached conceptual development plan and in the preliminary drainage report.

Should you have any questions, concerning the materials submitted, please contact me at 317-707-3708 or via email at SBrehob@Banning-eng.com

Sincerely,

A. Bull

Steven A. Brehob VP Development Services

Cc: Chad Ziegler Michael Carmin Tom Greeman



"Your Project is Our Priority"

PRELIMINARY DRAINAGE REPORT

FOR

GREEMAN STATE ROAD 446

LOCATED IN

MONROE COUNTY, INDIANA

Prepared for: Thomas Greeman PO Box 606 Seymour, IN 47274

July 1, 2024

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Drainage Narrative for: Greeman SR 446

I. INTRODUCTION

Greeman SR 446 is a proposed boat & trailer storage facility located on a $4.8\pm$ acre parcel on the west side of Indiana State Road 446 approximately 9.2 miles southeast of Indiana 46 in the southeast ¹/₄ of Section 16; Township 7 North; Range 1 East; Monroe County, Indiana. This report includes the result of a preliminary drainage analysis for the project, summarizing the allowable and proposed stormwater discharges and proposed stormwater detention.

II. ADJOINING LAND CONDITIONS

North: Undeveloped/Residential South: Undeveloped/Residential East: Undeveloped/Residential West: Undeveloped/Residential

III. EXISTING SOIL TYPES

| <u>Symbol</u> | Name | <u>Hydrologic</u> Soil Group |
|---------------|--|---------------------------------|
| BdB | Bedford silt loam, 2 to 6% | С |
| BkF | Brownstown-Gilwood silt loams, 25 to 75% | С |
| CrC | Crider silt loam, 6 to 12% | В |
| HaD | Hagerstown silt loam, 12 to 20% | С |
| TIB | Zanesville silt loam, 2 to 6% | С |
| WyqD | Wrays-Gilwood silt loams, 6 to 20% | С |

IV. SITE DRAINAGE ANALYSIS

a. Stormwater Discharge Requirements

The project is subject to the stormwater discharge requirements found in the Monroe County Stormwater Management Ordinance Chapter 761. See **TABLE 1** below.

TABLE 1STORMWATER DISCHARGE REQUIREMENTSMONROE COUNTY

| $Q_{10p} \leq 0.5$ cfs per ac. of development | Where: \mathbf{Q}_{10p} = Developed 10-year peak discharge rate |
|--|---|
| $Q_{100p} \leq 0.9$ cfs per ac. of development | Q_{100p} = Developed 100-year peak discharge rate |

b. Drainage Design

i. Existing Site Conditions

The existing site consists of mostly grass and wooded area bounded by a wooded embankment along the west side. There is a single-family home along with a mobile home and a barn located on the site. Most of the $4.8\pm$ acre site drains to the embankment into a ravine west of the site, which is considered the ultimate stormwater discharge point (BNDY) for the project.

ii. Developed Site Conditions

Approximately 2.1± acres of the site (area "DEV") will be developed. This includes the addition of a gravel parking area for storing boats & boat trailers. The existing home and barn will be removed, and the mobile home will remain as an office for the storage facility. Stormwater will be routed from east to west into a grass swale that drains to a dry bottom detention basin located in the southwest corner of the site. The detention basin will outlet to a shallow riprap basin that will function as a level spreader to reduce velocity and help convert the discharge from concentrated flow to sheet flow over the embankment. A hydrologic/hydraulic model of the developed site was completed to calculate the developed stormwater discharges and stage elevations. The total discharge from the developed project site to the boundary meets the allowable discharge found in stormwater technical standards manual. **Table 2** below provides a summary of the allowable and developed discharge rates for the project. **Table 3** provides a summary of the appendix.

| DEVE | <u>TABI</u> ELOPED SITE STOR | LE 2 MW | ATER DISCHARGE | |
|-----------------|-------------------------------------|------------|------------------------------|----|
| Discharge Point | Developed Onsite Discharge (CFS) | | Allowable Discharge (CFS) | |
| DNDV | $Q_{10p} = 1.1$ | ≤ | 2.1 ac x 0.5 cfs/ac = 1.1 | OK |
| DINDY | $Q_{100p} = 1.9$ | ≤ | 2.1 ac x 0.9 cfs/ac = 1.9 | OK |

| | <u>TABLE 3</u> DEVELOPED SITI DETENTION PONI | E DS |
|-----------------|--|--------------------------|
| Discharge Point | Outlet Control Elev. | 100-year Stage Elevation |
| POND | 796.5 | 800.6 |

The grass swale and the dry bottom pond will have a perforated subsurface drain to promote infiltration to function as a stormwater quality BMP. The swale will function as the first BMP with rock check dams that will reduce velocity and promote infiltration prior to reaching the detention basin. The detention basin will function as a second BMP with an additional infiltration function. Calculations will be provided with the final design.

In conclusion, the calculations in this report show that drainage from the project will not cause any adverse impacts to onsite or offsite facilities. We believe the project falls within the requirements of the applicable code of ordinances.

V. REFERENCES

Design data and methods are based on the following reference materials:

- 1. NRCS Web Soil Survey (<u>http://websoilsurvey.sc.egov.usda.gov/App/HomePage.htm</u>)
- 2. USGS Mapping
- 3. FEMA Flood Mapping (https://www.fema.gov/national-flood-hazard-layer-nfhl)
- 4. Monroe County drainage ordinance
- 5. Monroe County Stormwater Technical Standards Manual

A. AREA & SITE MAPS



Drawn By: WCZ Date: 6/27/2024 Project No.: 23288 Scale: NONE Page: 1 of 1 AREA MAP GREEMAN STATE ROAD 446 MONROE COUNTY BLOOMINGTON, INDIANA



PLAINFIELD, IN 46168 BUS: (317) 707-3700, FAX: (317) 707-3800 E-MAIL: Banning@BanningEngineering.com July W2D: www.BanningEngineering.com



Date: 6/27/2024 Project No.: 23288 Scale: NONE Page: 1 of 1

GREEMAN STATE ROAD 446 MONROE COUNTY **BLOOMINGTON, INDIANA**



B35 COLUMBIA ROAD, SUITE #107 PLAINFIELD, IN 46168 BUS: (317) 707-3700, FAX: (317) 707-3800 E-MAIL: Banning@BanningEngineering.com July W2D: www.BanningEngineering.com

B. SOILS MAP



Conservation Service

Hydrologic Soil Group-Monroe County, Indiana



Natural Resources Conservation Service

USDA

Hydrologic Soil Group

| Map unit symbol | Map unit name | Rating | Acres in AOI | Percent of AOI |
|---------------------------|--|--------|--------------|----------------|
| BdB | Bedford silt loam, 2 to 6 percent slopes | C/D | 11.0 | 15.8% |
| BkF | Brownstown-Gilwood silt loams, 25 to 75 percent slopes | C | 18.3 | 26.1% |
| CrC | Crider silt loam, 6 to 12 percent slopes | В | 19.6 | 28.0% |
| HaD | Hagerstown silt loam, 12 to 20 percent slopes | С | 0.3 | 0.5% |
| TIB | Zanesville silt loam, 2 to 6 percent slopes | C/D | 2.5 | 3.6% |
| WyqD | Wrays-Gilwood silt loams, 6 to 20 percent slopes | C | 18.1 | 25.9% |
| Totals for Area of Intere | est | | 69.8 | 100.0% |

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher



C. FLOOD MAP

National Flood Hazard Layer FIRMette

36°24'52"W 39°2'36"I





AREA OF MINIMAL FLOOD HAZARD

MONROEGOUNEY JEOM

18105C0275D

eff. 12/17

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

July 1, 2024

authoritative NFHL web services provided by FEMA. This map reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or The flood hazard information is derived directly from the was exported on 6/26/2024 at 8:23 AM and does not become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

Basemap Imagery Source: USGS National Map 2023 86°24'15"W 39°2'8" 1:6,000 ■ Feet 2,000 1,500 1,000 500 250 Page 13

D. DRAINAGE DESIGN

D1. DEVELOPED CONDITION BASIN MAP



D2. DEVELOPED CONDITION CALCULATIONS

Developed Condition Node Results

| Node Name | Sim Name | Warning Stage [ft] | Alert Stage [ft] | Max Stage [ft] | Min/Max Delta Stage | Max Infl | < Total ow [cfs] | Max Total Outflow | Max Surface Area [ft2] |
|-----------|----------|-----------------------|---------------------|-------------------|------------------------|-------------|---------------------|----------------------|---------------------------|
| | | | | | [ft] | | | [cfs] | |
| BNDY | 010Y-24H | 794.10 | 794.10 | 794.10 | 0.0000 | | 1.11 | 0.00 | 0 |
| BNDY | 100Y-24H | 794.10 | 794.10 | 794.10 | 0.0000 | | 1.89 | 0.00 | 0 |
| POND | 010Y-24H | 801.00 | 801.00 | 799.56 | -0.0010 | | 8.59 | 1.11 | 5832 |
| POND | 100Y-24H | 801.00 | 801.00 | 800.62 | 0.0010 | | 14.62 | 1.89 | 8859 |

Node Max Conditions : Multi Item | (name, sim) [Scenario1]

6/28/2024 11:57

D3. DEVELOPED CONDITION Tc AND Cn VALUES

| | -COMPOSITE CN- | TR-55 VALUES & TIM | E OF CONCEN | TRATION- | | | | | |
|-------|---|------------------------------------|-----------------|-----------------|-----|--|--|--|--|
| | PROJECT: JOB #: DATE: COMPUTED BY: | Greeman 23288 6/28/24 JAR | | | | | | | |
| | | | | | | | | | |
| | | BASINS | | | | | | | |
| BASIN | Composite CN | Area (ft ²) | Area (acres) | Area (miles) | Тс | | | | |
| DEV | 85.00 | 91476 | 2.10 | 0.00 | 6.0 | | | | |

| Basin | Cover Description | Soil Group | Area (Acres) | CN | CN * Acres | number | Basin # | CN | Acres | CN * Acres |
|-------|--------------------------------------|------------|--------------|----|------------|--------|---------|----|-------|------------|
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | i | | | |
| | | | | | | | DEV | 85 | 2.1 | 179.3 |
| DEV | Gravel (w/ right-of-way) | U | 0.7 | 89 | 62.3 | 4 | | | | |
| DEV | Gravel (w/ right-of-way) | ۵ | 0.4 | 91 | 36.4 | S | | | | |
| DEV | Paved parking lots, roofs, driveways | ۵ | 0.1 | 98 | 9.8 | 5 | | | | |
| DEV | Good condition; grass cover > 75% | U | 0.2 | 74 | 14.8 | 4 | | | | |
| DEV | Good condition; grass cover > 75% | D | 0.7 | 80 | 56 | £ | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |

| | | | | | | | | | | - | 9 |
|-------------------------|---------------|----------|------------|------------|---------|---------|----------------------|-----------|---------------------|----------|----------------------------|
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| | | | nour rain | = | | | | thanne | r | (ft) | |
| Sreeman 23288 | | | 2 year, 24 | ninimum T_ | | | | 0 | Ρw | (ft) | |
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| PROJ J | | | | | | | | | Length | (ft) | 275 |
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| | | | | | | | | Shallow | Length | (ft) | 230 |
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| et | | | 0.0 | 0.0 | 0.0 | | | I Flow | n Vel |) (ft/s | Р 3.2 |
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| ion We | l's n | hannel F | | | | | | oncer | s S | % (| 2.5 |
| oncentrat ed on TR-{ | : for Manning | C | grass | concrete | rip-rap | | | Shallow (| Length | (ft) | 190 |
| le of C Base | al values | | 0.15 | 0.24 | 0.011 | 0.4 | 0.17 | | T_t | (min) | 00700 |
| Tim | Typic | d Flow | | | | | % | | ч | | 0.01 |
| | | Overlan | ass | Irass | ≱nt | ight | ∋d > 20 ^c | id flow | S | % | 2.5 |
| | | | short gr | dense g | paveme | woods l | cultivate | Overlan | Length | (ft) | 100 |
| | | | | | | | | | Basin | name | ∧ <u>-</u> |
| | | | | | | | | | | | DE |

D4. DEVELOPED CONDITION ICPR INPUT DATA



Simple Basin: DEV

| Scenario: | Scenario1 |
|------------------------|----------------------|
| Node: | POND |
| Hydrograph Method: | NRCS Unit Hydrograph |
| Infiltration Method: | Curve Number |
| Time of Concentration: | 6.0000 min |
| Max Allowable Q: | 9999.00 cfs |
| Time Shift: | 0.0000 hr |
| Unit Hydrograph: | UH484 |
| Peaking Factor: | 484.0 |
| Area: | 2.1000 ac |
| Curve Number: | 85.0 |
| la/S: | 0.00 |
| % Impervious: | 0.00 |
| % DCIA: | 0.00 |
| % Direct: | 0.00 |
| Rainfall Name: | |

Comment:

Node: BNDY

| Scenario: | Scenario1 |
|-----------------|------------|
| Type: | Time/Stage |
| Base Flow: | 0.00 cfs |
| Initial Stage: | 794.00 ft |
| Warning Stage: | 794.10 ft |
| Alert Stage: | 794.10 ft |
| Boundary Stage: | |
| | |

| Year | Month | Day | Hour | Stage [ft] |
|------|-------|-----|---------|------------|
| 0 | 0 | 0 | 0.0000 | 794.10 |
| 0 | 0 | 0 | 24.0000 | 794.10 |

Comment:

Node: POND

| Stage [ft] | Area [ac] | Area [ft2] |
|------------|-----------|------------|
| 796.50 | 0.0000 | 0 |
| 797.00 | 0.0300 | 1307 |
| 798.00 | 0.0620 | 2701 |
| 799.00 | 0.1000 | 4356 |
| 800.00 | 0.1600 | 6970 |
| 801.00 | 0.2300 | 10019 |

Comment:

| Drop Structure Link · POND-BN | DY | Unstrea | am Pine | Downstr | am Pine |
|-------------------------------|------------------------|--------------|------------------|---|-------------|
| Scenario: Scen | hario1 | Invert. | 794 50 ft | Invert | 794 30 ft |
| From Node: PON | ID | Manning's N: | 0.0130 | Manning's N: | 0.0130 |
| To Node: BND | γ | Geometry | r: Circular | Geometry | r: Circular |
| Link Count: 1 | | Max Depth: | 1.00 ft | Max Depth: | 1.00 ft |
| Pipe Flow Direction: Both | 1 | | Bottom Cli | 0 | |
| Solution: Com | bine | Default: | 0.00 ft | Default: | 0.00 ft |
| Increments: 0 | | Op Table: | | Op Table: | |
| Pipe Count: 1 | | Ref Node: | | Ref Node: | |
| Damping: 0.00 | 000 ft | Manning's N: | 0.0000 | Manning's N: | 0.0000 |
| Length: 50.0 | 00 ft | | Top Clip | 3 | |
| FHWA Code: 0 | | Default: | 0.00 ft | Default: | 0.00 ft |
| Entr Loss Coef: 0.50 |) | Op Table: | | Op Table: | |
| Exit Loss Coef: 0.00 |) | Ref Node: | | Ref Node: | |
| Bend Loss Coef: 0.00 |) | Manning's N: | 0.0000 | Manning's N: | 0.0000 |
| Bend Location: 0.00 |) dec | | | | |
| Energy Switch: Ener | av | | | | |
| Pipe Comment: | 37 | | | | |
| | | | | | |
| Weir Co | omponent | | | | |
| Weir: | 1 | | Botto | om Clip | |
| Weir Count: | 1 | | Default: | 0.00 ft | |
| Weir Flow Direction: | Both | | Op Table: | | |
| Damping: | 0.0000 ft | | Ref Node: | | |
| Weir Type: | Sharp Crested Vertical | | Тор | o Clip | |
| Geometry Type: | Circular | | Default: | 0.00 ft | |
| Invert: | 796.50 ft | | Op Table: | | |
| Control Elevation: | 796.50 ft | | Ref Node: | | |
| Max Depth: | 0.42 ft | | Discharge | Coefficients | |
| | | | Weir Default: | 3.200 | |
| | | | Weir Table: | | |
| | | | Orifice Default: | 0.600 | |
| | | | Orifice Table: | | |
| Weir Comment: | | | | | |
| | | | | | |
| Weir Co | omponent | _ | | | |
| Weir: | 2 | | Botto | | |
| Weir Count: | 1 | | Default: | 0.00 ft | |
| weir Flow Direction: | Both | | Op Table: | | |
| Damping: | 0.0000 ft | | Ref Node: | | |
| vveir Type: | Sharp Crested Vertical | | 10j | | |
| Geometry Type: | | | Derault: | 0.00 11 | |
| Invert: | 799.60 ft | | Op Table: | | |
| Control Elevation: | 799.60 IL | | Rei Node: | O = - ff ' = i = - + - | |
| Max Depth: | U.42 II | | Discharge | 2 200 | |
| | | | | 3.200 | |
| | | | | 0.600 | |
| | | | | 0.000 | |
| Weir Comment: | | | Office Table: | | 1 |
| | | | | | |
| Dron Structure Comment | | | | | |
| | | | | | |


Dflt Damping (1D): 0.0050 ft Min Node Srf Area 100 ft2 (1D): Energy Switch (1D): Energy

Comment:

| 8 | | | | |
|-----------------------|-----------------------|--------------------------|------------------------|----------------------|
| | | | | |
| Simulation: 010Y-24H | | | | |
| Scenario: | Scenario1 | | | |
| Run Date/Time: | 6/28/2024 11:56:54 AM | | | |
| Program Version: | StormWise 4.08.00 | | | |
| | | | | |
| | | General | | |
| Run Mode: | Normal | | | |
| | | | | |
| | Year | Month | Day | Hour [hr] |
| Start Time: | 0 | 0 | 0 | 0.0000 |
| End Time: | 0 | 0 | 0 | 24.0000 |
| | Livelande av (Fee el | | | |
| | Hydrology [sec] | | | |
| Min Calculation Time: | 60,0000 | [Set] | - | |
| Max Calculation Time: | 00.0000 | 20,0000 | | |
| | | 30.0000 | | |
| | | Output Time Increments | | |
| | | | | |
| Hydr | ology | | | |
| | | • | | |
| Year | Month | Day | Hour [hr] | Time Increment [min] |
| 0 | 0 | 0 | 0.0000 | 15.0000 |
| | | | | |
| Surface H | Hydraulics | | | |
| Veen | Manda | Davi | | Time Incoment [min] |
| Year | Month | Day | Hour [nr] | |
| 0 | 0 | 0 | 0.0000 | 15.0000 |
| Resta | art File | I | | |
| Save Restart | False | 1 | | |
| | | | | |
| | | Resources & Lookup Table | S | |
| | | | | |
| Reso | ources | | Lookup | Tables |
| Rainfall Folder: | | - | Boundary Stage Set: | |
| | | | Extern Hydrograph Set: | |
| Unit Hydrograph | | | Curve Number Set: | |
| Folder: | | | | |
| | | | Green-Ampt Set: | |
| | | | Vertical Layers Set: | |
| | | | Impervious Set: | |
| | | | | |
| | | Tolerances & Options | | |
| Time Marching | SVOD | | IA Decovery Time | 24 0000 br |
| Max Iterations: | SAUR 6 | | TA RECOVELY TIME: | 24.0000 III |
| wax rierations: | U | | | |

| Over-Relax Weight | 0.5 dec | Ia/S: | 0.20 dec |
|---------------------|-----------|---------------------|------------|
| Fact: | | | |
| dZ Tolerance: | 0.0010 ft | | |
| Max dZ: | 1.0000 ft | Smp/Man Basin Rain | Global |
| | | Opt: | |
| Link Optimizer Tol: | 0.0001 ft | | |
| | | Rainfall Name: | ~SCSII-24 |
| | | Rainfall Amount: | 4.44 in |
| | | Storm Duration: | 24.0000 hr |
| | | Dflt Damping (1D): | 0.0050 ft |
| | | Min Node Srf Area | 100 ft2 |
| | | (1D): | |
| | | Energy Switch (1D): | Energy |
| | | | |

Comment:

| Simulation: 100Y-24H | | | | |
|-----------------------|-----------------------|--------------------------|------------------------|----------------------|
| Scenario: | Scenario1 | | | |
| Run Date/Time: | 6/28/2024 11:57:02 AM | | | |
| Program Version: | StormWise 4.08.00 | | | |
| | | _ | _ | |
| | | General | | |
| Run Mode: | Normal | | | |
| | | | 5 | |
| | Year | Month | Day | Hour [hr] |
| Start Time: | 0 | 0 | 0 | 0.0000 |
| End Time: | 0 | 0 | 0 | 24.0000 |
| | Lludrology [cool] | Surface Undraulias | | |
| | nyululuyy [sec] | | | |
| Min Calculation Time: | 60,0000 | 0 1000 | - | |
| Max Calculation Time: | 00.0000 | 30,0000 | | |
| | | 30.0000 | | |
| | | Output Time Increments | | |
| | | | | |
| Hydr | ology | | | |
| | | - | | |
| Year | Month | Day | Hour [hr] | Time Increment [min] |
| 0 | 0 | 0 | 0.0000 | 15.0000 |
| | | | | |
| Surface F | lydraulics | | | |
| N | | D | | |
| Year | Month | Day | Hour [hr] | Time Increment [min] |
| 0 | 0 | 0 | 0.0000 | 15.0000 |
| Docta | rt Filo | 1 | | |
| Save Restart: | Falso | 1 | | |
| Save Restart. | T disc | | | |
| | | Resources & Lookup Table | S | |
| | | | | |
| Reso | urces | | Lookup | Tables |
| Rainfall Folder: | | • | Boundary Stage Set: | |
| | | | Extern Hydrograph Set: | |
| Unit Hydrograph | | | Curve Number Set: | |

Folder:

Green-Ampt Set: Vertical Layers Set: Impervious Set:

| | | Tolerances & Options | |
|---------------------|-----------|----------------------|------------|
| Time Marching: | SAOR | IA Recovery Time: | 24.0000 hr |
| Max Iterations: | 6 | | |
| Over-Relax Weight | 0.5 dec | la/S: | 0.20 dec |
| Fact: | | | |
| dZ Tolerance: | 0.0010 ft | | |
| Max dZ: | 1.0000 ft | Smp/Man Basin Rain | Global |
| | | Opt: | |
| Link Optimizer Tol: | 0.0001 ft | | |
| | | Rainfall Name: | ~SCSII-24 |
| | | Rainfall Amount: | 6.80 in |
| | | Storm Duration: | 24.0000 hr |
| | | Dflt Damping (1D): | 0.0050 ft |
| | | Min Node Srf Area | 100 ft2 |
| | | (1D): | |
| | | Energy Switch (1D): | Energy |
| | | | |
| Comment: | | | |





MONROE COUNTY HIGHWAY DEPARTMENT

501 N Morton St. Suite 216, Bloomington IN 47404 • (812) 349-2565 • www.co.monroe.in.us

To:Monroe County Drainage BoardFrom:Kelsey Thetonia, MS4 Coordinator

Date: 8/9/2024

Re: Fry Rd. Residential Project Drainage Plan

I am seeking your input on a residential project on Fry Rd. The owners will be adding a large addition to the current residence on the property, as well as adding a new garage and barn. The property contains a stream with a drainage area greater than 1 square mile with unmapped floodplain. At the County's request, they have submitted a request to DNR to have this floodplain mapped and we are still awaiting those results.

The configuration of the new garage will require disturbance and relocation of two ephemeral streams on the property. Their engineer submitted a drainage plan where they are proposing rerouting these ephemeral channels. I would like your input on this proposed plan and how this works with our new ordinance.



MEMORANDUM:

| То: | Kelsey Thetonia, MS4 Coordinator, Monroe County Drainage Board |
|-------------------|---|
| | bound |
| From: | Katie Stein, PE |
| Subject: | 601 Fry Road |
| Date: | July 30, 2024 |
| Subject: Date: | 601 Fry Road July 30, 2024 |

<u>Summary</u>

The residents of 601 W Fry Road are expanding their existing home, constructing a new detached garage and new barn within their property. The homeowners would like to locate the barn and garage in specific areas based on their vision and use of the two buildings. The garage location specifically is proposed within an existing drainage swale. The owners would like to relocate the drainage swale in order to construct the garage in their desired location.

Existing Conditions

There are two drainage swales in the proposed garage location that converge just east of the proposed garage location. The upstream drainage area of the western portion is approximately 5.20 acres and the eastern area is approximately 2.27 acres. Both areas are heavily wooded and steep. The western drainage and swale are of most concern in relation to the garage.

For this project, the rational method was used to determine the approximately existing flow rates for both swales. Due to the steepness and type of flow (concentrated/channel) of the upstream drainage areas, both drainage areas had a Time of Concentration of approximately 5 minutes and below is a summary of the approximate flow rates for the Q100:

| Drainage | Drainage Area | Runoff | Q100 |
|--|---------------|--------------|-----------|
| Area | | Coefficient* | Flow Rate |
| | Acre | | cfs |
| Western | 5.20 | 0.83 | 44.83 |
| Eastern | 2.27 | 0.83 | 19.57 |
| Combined | | | 64.4 |
| *From Table 2-2 - Woodland (Clay) Steep, Greater than 7% slope | | | |

of the Monroe County Stormwater Technical Standards Manual

Proposed Conditions

The proposed plan is for a new swale relocation to be constructed on the north side the proposed garage. Below is the typical swale geometry proposed:



The recommendation treatment for the bottom of the swale is rip rap due to the velocity (~10-14 fps) of stormwater. Due to this swale being on the north side of the garage, it may be difficult to establish vegetation within the swale and side slopes and therefore rip rap is recommended on the side slopes as well. However, if vegetation is able to be established to stabilize the side slope and even bottom of the swale without causing erosion, vegetation could be used.

Attachments

Schematic Drainage Plan (Separate) Drainage Area Calculations Hydraflow Hydrographs Results Swale Capacity Calculations

Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Tuesday, 07 / 30 / 2024

Hyd. No. 2

West Area

| Hydrograph type | = Rational | Peak discharge | = 19.57 cfs |
|-----------------|---------------------|-------------------|--------------|
| Storm frequency | = 100 yrs | Time to peak | = 5 min |
| Time interval | = 1 min | Hyd. volume | = 5,871 cuft |
| Drainage area | = 2.270 ac | Runoff coeff. | = 0.83 |
| Intensity | = 10.387 in/hr | Tc by TR55 | = 5.00 min |
| IDF Curve | = Monroe County.IDF | Asc/Rec limb fact | = 1/1 |
| | | | |



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Tuesday, 07 / 30 / 2024

Hyd. No. 2

East Area

| Hydrograph type | = Rational | Peak discharge | = 19.57 cfs |
|-----------------|---------------------|-------------------|--------------|
| Storm frequency | = 100 yrs | Time to peak | = 5 min |
| Time interval | = 1 min | Hyd. volume | = 5,871 cuft |
| Drainage area | = 2.270 ac | Runoff coeff. | = 0.83 |
| Intensity | = 10.387 in/hr | Tc by TR55 | = 5.00 min |
| IDF Curve | = Monroe County.IDF | Asc/Rec limb fact | = 1/1 |
| | | | |



Hydrograph Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2023

Tuesday, 07 / 30 / 2024

Hyd. No. 3

Total East

| Hydrograph type | = Combine | Peak discharge | = 64.40 cfs |
|-----------------|-----------|----------------------|---------------|
| Storm frequency | = 100 yrs | Time to peak | = 5 min |
| Time interval | = 1 min | Hyd. volume | = 19,320 cuft |
| Inflow hyds. | = 1, 2 | Contrib. drain. area | = 7.470 ac |
| Time interval | = 1 min | Hyd. volume | = 19,320 cuft |
| Inflow hyds. | = 1, 2 | Contrib. drain. area | = 7.470 ac |





 $R = \frac{A}{P}$









MONROE COUNTY HIGHWAY DEPARTMENT

501 N Morton St. Suite 216, Bloomington IN 47404 • (812) 349-2565 • www.co.monroe.in.us

- To: Monroe County Drainage Board
- From: Kelsey Thetonia, MS4 Coordinator
- Date: 8/9/2024
- Re: 6251 W Ison Rd. Enforcement Case

I am seeking your input on an ongoing enforcement case we received from the Planning Department. The owner of the property at 6251 W Ison Rd. has constructed a parking lot for storage of trucks, materials, and equipment. The parking area and additional fill is located within a sinkhole and Karst Conservancy Area.

I am asking the Drainage Board to determine if the parking area/fill should be removed and restored to vegetation, or left in place with no further grading or impact to the sinkhole.

1211 S Walnut St Bloomington, IN 47401

Brian Burke 6251 W Ison Rd Bloomington, IN 47404

Subject: 6251 W Ison Rd – Karst Survey Bloomington, IN Date: June 3, 2024

Contact: Jason Krothe

Phone: 812-219-0210

Email: jnkrothe@hydrogeologyinc.com

Mr. Burke,

Hydrogeology Inc. (HGI) respectfully submits this summary report for the karst survey conducted at 6251 W Ison Rd in Bloomington, IN (the Site, Figure 1).

1 – Overview

The Site is located at 6251 W Ison Rd in Bloomington, Indiana and is approximately 12acres (Figure 2). The purpose of this survey was to identify karst features at the Site. The Site consists of mostly grassland with a gravel driveway and parking area.

2 - Geology / Physiography

The Site is in the Crawford Upland physiographic region, which is one of the primary karst forming areas in Indiana. The bedrock at the Site is the Ste. Genevieve Limestone (Hasenmueller, Estell, Keith, and Thompson, 2008). The Ste. Genevieve Limestone is composed of primarily limestone with small amounts of shale, dolomite, sandstone, and chert (Carr, Leininger, and Golde, 1978).

3 – Sinkholes & Springs

Sinkholes are surface depressions that form in a variety of ways in karst areas (Figure 3). Sinkholes will often time have a swallow hole, which is an opening in the ground where water infiltrates. Groundwater flow in karst areas is predominantly fracture flow, meaning the bedrock itself has low permeability while the fractures in the bedrock are open conduits that allow water, soil, and other materials to travel quickly through the subsurface. Water that drains into a sinkhole can eventually discharge at a karst spring (Figure 4).



1211 S Walnut St Bloomington, IN 47401

4 – Karst Desktop Review

A review of available karst resources was conducted prior to the field survey. Those resources include United State Geological Survey (USGS) topographic maps, Indiana Map 2011 1-ft LIDAR, karst spring maps, and private cave databases. Three sinkholes were identified from LIDAR data and topographic maps (Figure 5).

5 – Karst Field Survey

HGI conducted a field review of the Site on May 24, 2024. No new karst features were located at the Site during the field survey. Field photographs can be seen in Appendix A. The three sinkholes identified in the desktop review are described below:

SH-01 – SH-01 is approximately 1000 feet long, 870 feet wide, and 12 feet deep. It encompasses 15 acres although only approximately 4.5 acres fall within the property boundary of the Site. The sinkhole has well developed grass and wildflowers within it. No bedrock or surface openings were observed within the portion of the sinkhole on the Site.

SH-01 Mitigation – SH-01 should receive a sinkhole conservancy area (SCA) in accordance with Monroe County Planning guidelines. The location of the SCA should be based on a Site survey. The gravel driveway and parking area at the Site encroach on approximately 0.10 acres of the sinkhole. No impacts from the driveway or parking area to the sinkhole were observed during the field review. SH-01 should be protected with erosion and sediment control measures during any future development near the sinkhole.

SH-02 – SH-02 is approximately 2000 feet long, 1400 feet wide, and 21 feet deep. It encompasses 39 acres although only approximately 1.2 acres fall within the property boundary of the Site. The sinkhole has well developed grass and wildflowers within it. No bedrock or surface openings were observed within the portion of the sinkhole on the Site.

SH-02 Mitigation – SH-02 should receive an SCA in accordance with Monroe County Planning guidelines. The location of the SCA should be based on a Site survey. SH-02 should be protected with erosion and sediment control measures during any future development near the sinkhole.

SH-03 – SH-03 is approximately 200 feet in diameter and 4 feet deep. SH-03 encompasses 0.7 acres and has well-developed grass within it. No bedrock or surface openings were observed within the sinkhole. Some areas of disturbed soil and mounded bare soil are present within the sinkhole. The parking area at the Site encroaches on approximately 0.01 acres of the sinkhole. No impacts from the parking

1211 S Walnut St Bloomington, IN 47401

area to the sinkhole were observed during the field review. An earthen mound has been installed on the east side of the sinkhole. There are no obvious signs this mound has impacted the sinkhole or the adjacent property.

SH-03 Mitigation – SH-03 should receive an SCA in accordance with Monroe County Planning guidelines. The location of the SCA should be based on a Site survey. The parking area at the Site encroaches on approximately 0.01 acres of the sinkhole. No impacts from the parking area to the sinkhole were observed during the field review. The areas of disturbed soil within SH-03 should be graded and seeded to prevent impacts to the sinkhole. SH-03 should be protected with erosion and sediment control measures during any future development near the sinkhole.

6 – Study Limitations

The identification of karst features at the Site was limited to surface inspection. No subsurface investigations were conducted. Undocumented karst features are possible in the subsurface.

7 – Summary

A desktop review and field survey were conducted at the Site to identify karst features. Three sinkholes are present at the Site. All three sinkholes should receive an SCA and should be protected with erosion and sediment control measures during future development. There are no visible impacts from the existing driveway or parking area on any of the sinkholes. The field survey was limited to surface inspection with no subsurface investigation. Unknown karst features are possibly present in the subsurface at the Site. If a previously unknown karst feature is discovered during construction activities the feature should be protected with erosion and sediment control measures and inspected by a karst specialist.

hydrogeology inc.

1211 S Walnut St Bloomington, IN 47401

HGI appreciates the opportunity to provide this summary report. If you have any questions, concerns, or comments please do not hesitate to contact me directly at (812) 219-0210.

Sincerely,

Hydrogeology Inc.

Jason N. Krothe, LPG IN-2511 President



hydrogeology inc.

1211 S Walnut St Bloomington, IN 47401

References

Carr, D. D., Leininger, R. K., and Golde, M. V., 1978, Crushed stone resources of the Blue River Group (Mississippian) of Indiana: Indiana Geological Survey Bulletin 52, 225 p.

Hasenmueller, W. A., Estell, C. M., Keith, B., and Thompson, T. A., 2009, Bedrock geologic map of Monroe County, Indiana: Indiana Geological Survey Miscellaneous Map 73, scale 1:48,000.

















Photograph Number:

2

Coordinates (UTM Meters) NA

INA

Photograph Date: 5-24-24

Comments:

South side of the Site looking at SH-03.





Comments: Center of the Site looking west.







West side of the Site looking north.



Karst Survey - 6251 W Ison Rd Appendix A





Photograph Number:

10

Coordinates (UTM Meters)

NA

Photograph Date: 5-24-24

Comments:

Center of the Site looking at SH-01.



West side of the Site looking

Recommended treatment:

south.

NA









Karst Survey - 6251 W Ison Rd Appendix A



Karst Survey - 6251 W Ison Rd Appendix A



Northwest corner of the Site looking southeast.




Comments: North side of the Site looking at SH-01.











Comments:

East side of the Site looking south.



| Photograph Number: 27 | |
|---|--|
| Coordinates (UTM Meters) | |
| NA | |
| Photograph Date: 5-24-24 | |
| Comments: East side of the Site looking south. | |
| | |
| Recommended treatment: NA | |
| Photograph Number: | |
| 28 | |
| Coordinates (UTM Meters) NA | |
| Photograph Date: 5-24-24 | |
| Comments: Disturbed soil on the edge of SH-03. | |
| | |
| | |

Recommended treatment: NA

Page 14 of 18

Photograph Number: 29 Coordinates (UTM Meters) NA Photograph Date: 5-24-24 Comments: Mound along east side of SH-03. Recommended treatment: NA



Photograph Number: 30 Coordinates (UTM Meters) NA Photograph Date: 5-24-24 Comments: Disturbed soil on the edge of SH-03. Recommended treatment: NA







Photograph Number:

32

Coordinates (UTM Meters)

NA

Photograph Date: 5-24-24

Comments:

Disturbed soil on the edge of SH-03.





34 Coordinates (UTM Meters) NA Photograph Date: 5-24-24

Comments: Looking at SH-03.



Karst Survey - 6251 W Ison Rd Appendix A

Photograph Number: 35 Coordinates (UTM Meters) NA Photograph Date: 5-24-24 Comments: Disturbed soil on the edge of SH-03. Recommended treatment: NA



Photograph Number:

36

Coordinates (UTM Meters)

NA

Photograph Date: 5-24-24

Comments:

Center of the site looking northeast at SH-01.





1 2 blobs were mistaker

Site Plan from 2016 Grading Permit issued by Planning Department



eventulally purpose

house and garage Later. Rold now just trying to get mess cleaned up that previous owner left